

HDR

# **Load Rating Challenges in Idaho**

Parking Garages under Public Roads

and

The Wallace Viaduct

Scott Wood, P.E. & Will Johnson, P.E.



**BOISE**

**BOI ZEE**



**BOI SEE**

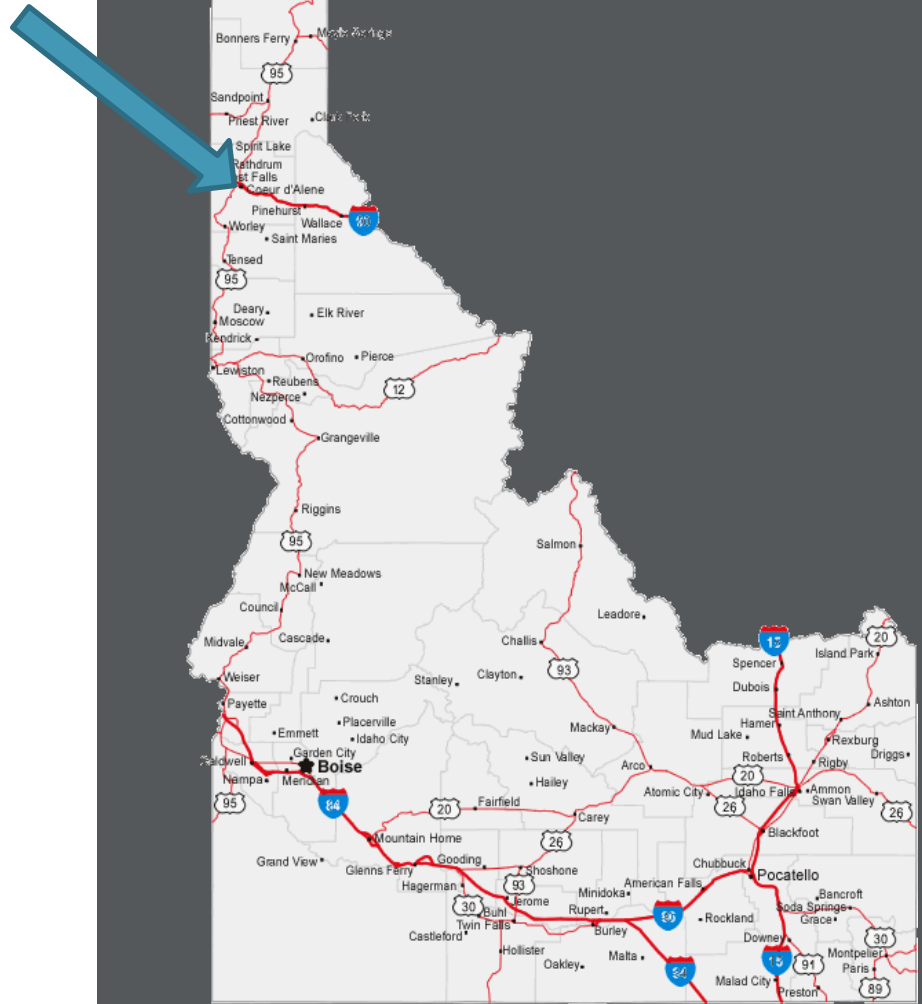


**WHEN YOU DON'T EVEN  
KNOW IT'S THERE**



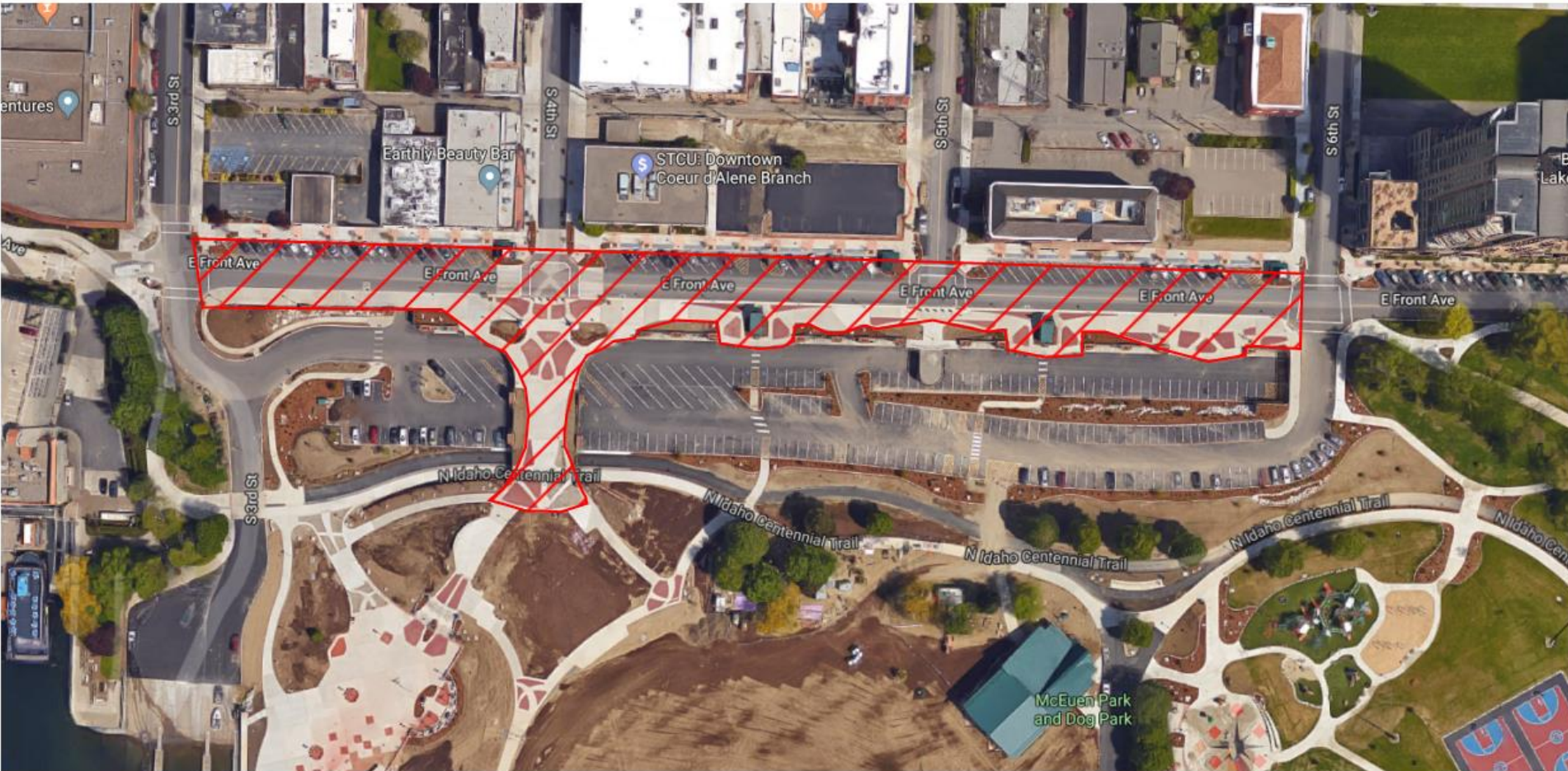
# McEuen Park Garage in Coeur d'Alene, Idaho

- Owner: City of Coeur d'Alene
- Opened in 2014
- 20-acre, \$20 million project
- Companies/names withheld

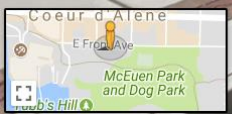




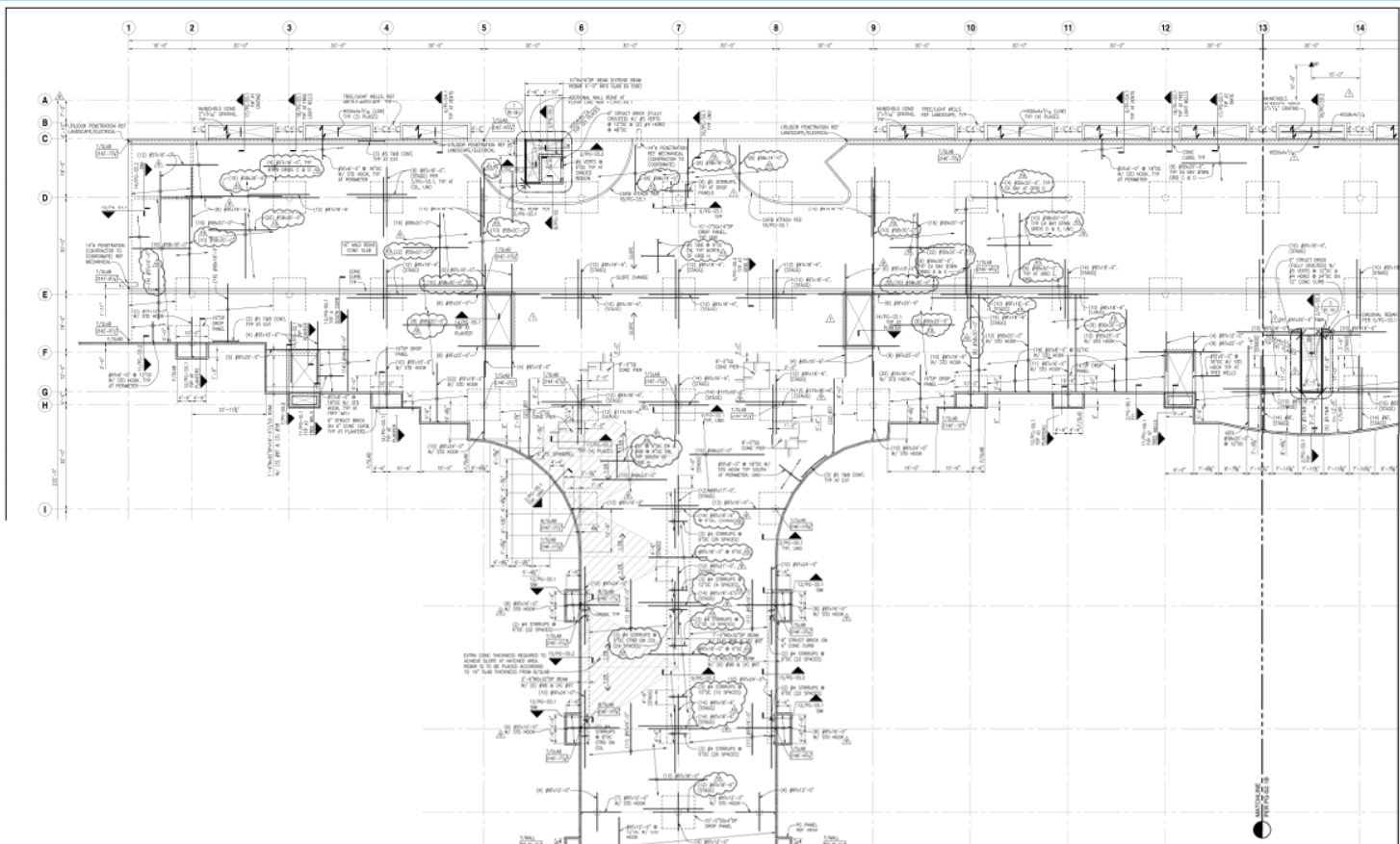











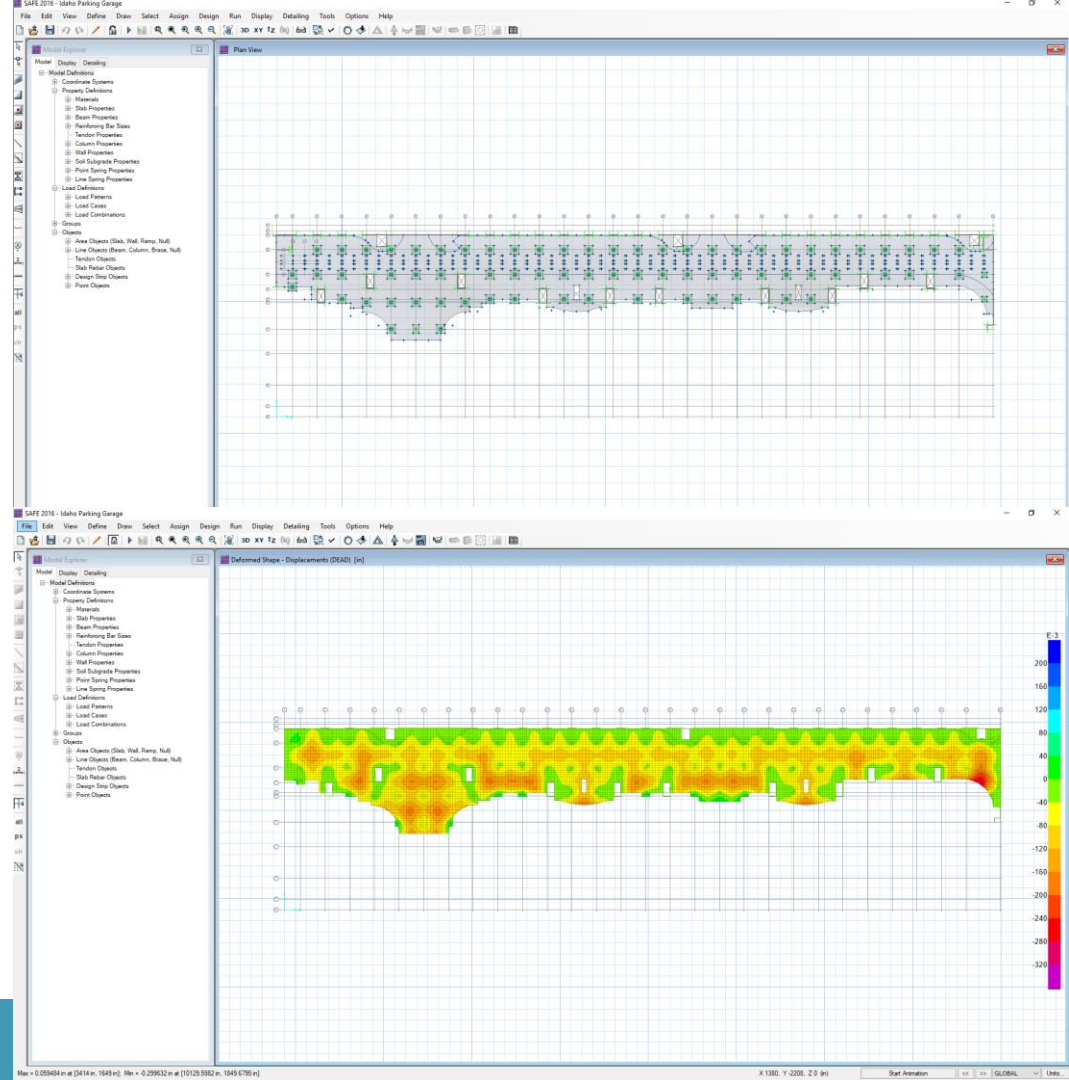


LEVEL 2 FRAMING PLAN  
SCALE: 1/8" = 1'-0"

 <b>McEuen</b> <b>PARK</b> IMPROVEMENTS City of Cedar Rapids Cedar Rapids, IA 52404	<b>Team</b> <b>McEuen</b> <small>3000 W. Grand Avenue, Suite 200          Cedar Rapids, IA 52404          P: 319.244.1777   F: 319.244.5114</small>		<table border="1"> <thead> <tr> <th>NO.</th> <th>REVISION</th> <th>DATE</th> <th>BY</th> </tr> </thead> <tbody> <tr> <td>1</td> <td>ISSUE FOR PERMIT</td> <td>02-14-13</td> <td>JL</td> </tr> <tr> <td>2</td> <td>REVISED PER COMMENTS</td> <td>02-28-13</td> <td>JL</td> </tr> <tr> <td>3</td> <td>REVISED PER COMMENTS</td> <td>03-05-13</td> <td>JL</td> </tr> <tr> <td>4</td> <td>REVISED PER COMMENTS</td> <td>04-24-13</td> <td>JL</td> </tr> <tr> <td>5</td> <td>REVISED PER COMMENTS</td> <td>04-26-13</td> <td>JL</td> </tr> </tbody> </table>	NO.	REVISION	DATE	BY	1	ISSUE FOR PERMIT	02-14-13	JL	2	REVISED PER COMMENTS	02-28-13	JL	3	REVISED PER COMMENTS	03-05-13	JL	4	REVISED PER COMMENTS	04-24-13	JL	5	REVISED PER COMMENTS	04-26-13	JL
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<b>LEVEL 2 FRAMING PLAN</b> <b>AREA A</b>			2013 McEuen Construction Project DESIGNED BY: <b>MSA</b> DRAWN BY: <b>SSD</b> CHECKED BY: <b>SSD</b> DATE: 04-26-13																								

# RATING PROGRAM

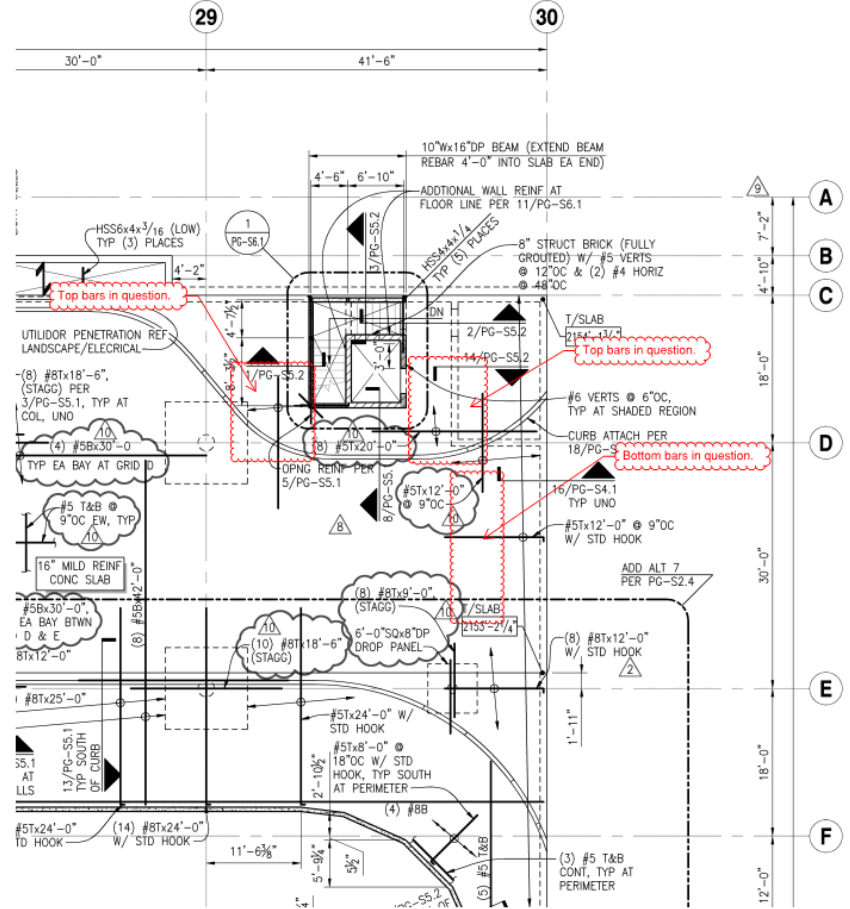
CSI SAFE





# RATING ISSUES

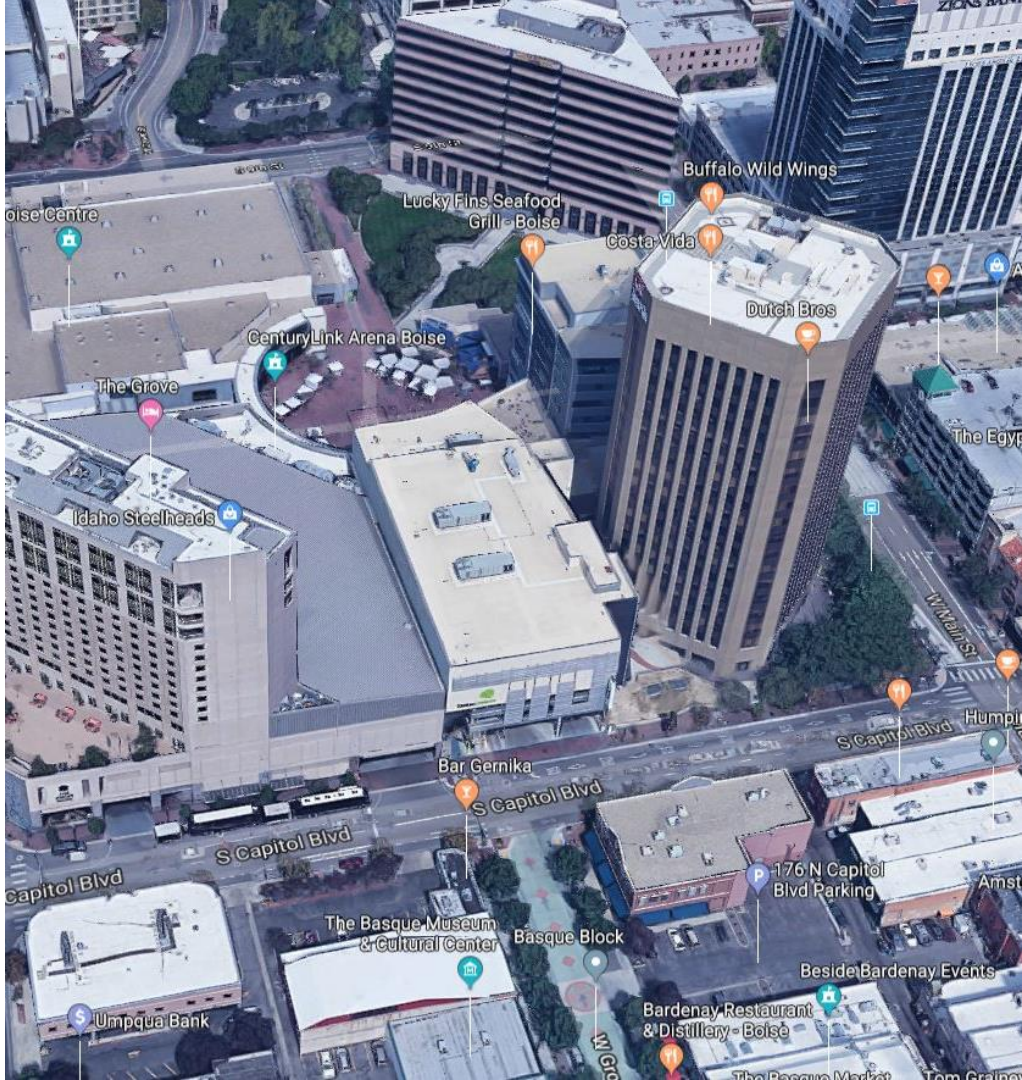
- Two-Way Slab
- DRAWINGS (Architectural, Structural & Civil)
- CSI SAFE
- Live Loading
- Pre/Post Processing
- Design Error?



Problem Areas Screenshot

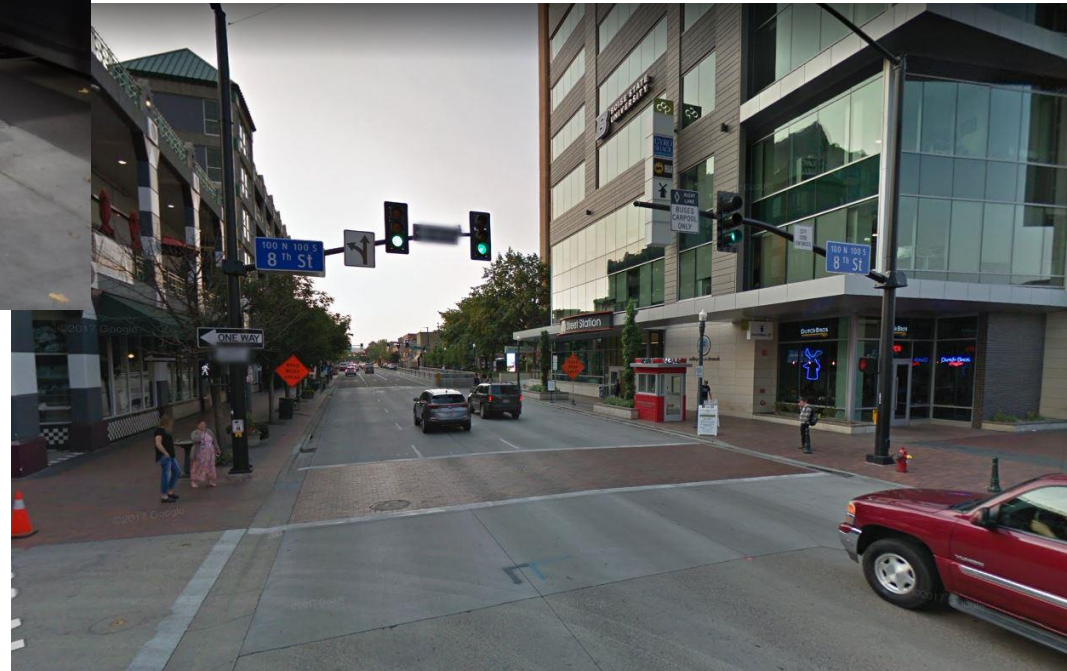
# Main Street in Boise, Idaho

- Owner: ??????????
- Valley Regional Transit / Ada County HD/  
Private Developer
- Opened in 2016



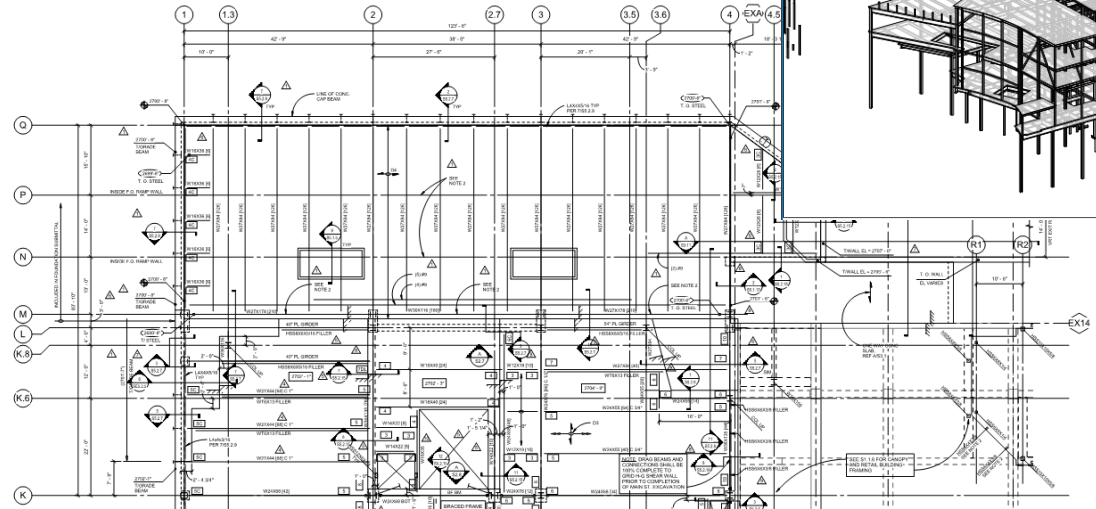
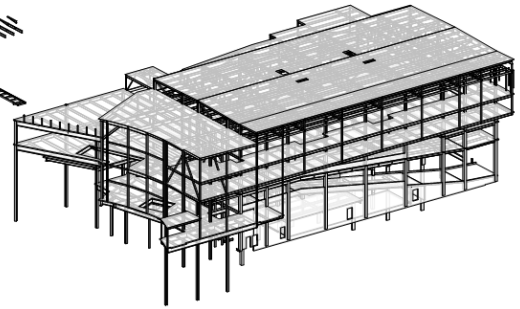
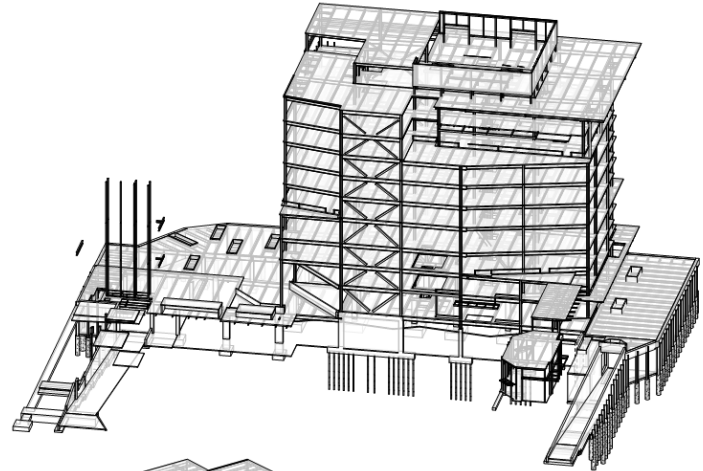






# DESIGN PLANS

- Many, many sheet
- Many, many Details
- Architectural, Structural, & Civil



CITY CENTER PLAZA  
GARDNER COMPANY  
777 MARKET STREET, SUITE 2000  
SAN FRANCISCO, CA 94102  
ARCHITECT  
CITY CENTER PLAZA  
777 MARKET STREET, SUITE 2000  
SAN FRANCISCO, CA 94102  
ARCHITECT  
3D VIEWS

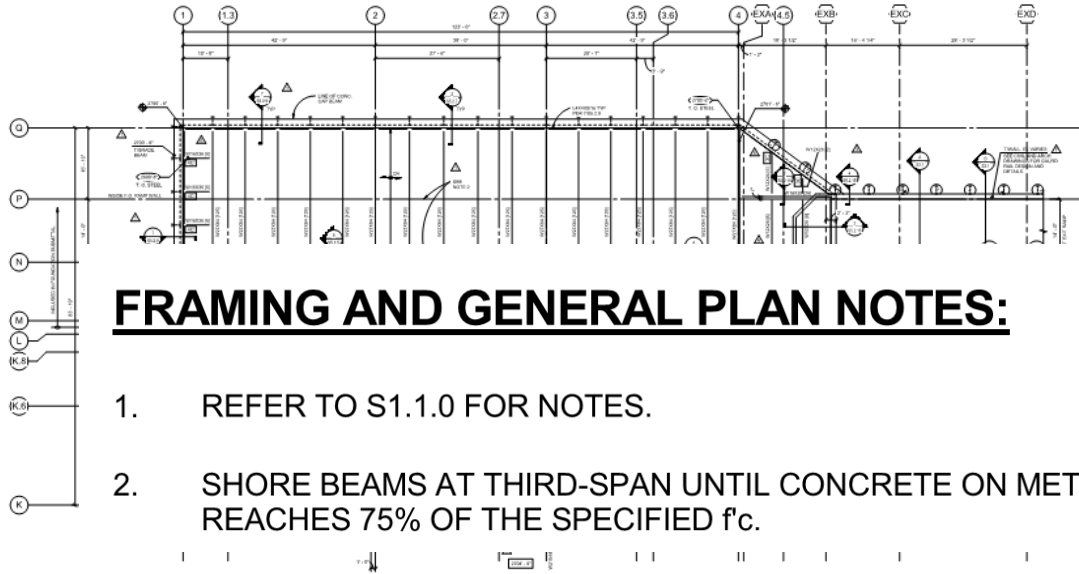
# BrR RATING

- Floor Beam / Stringer

The image displays two windows from a structural analysis software interface. The top window, titled "Schematics: Framing Plan View", shows a plan view of a bridge with three spans. It features a toolbar at the top with various icons and a status bar indicating "US Customary" units and a "Preliminary" status. The main area shows a grid of vertical lines representing girders, with two lines labeled "Girder 1" and "Girder 2". The bottom window, titled "Schematics: Bridge Typical Cross Section View", shows a cross-section of the bridge deck. It includes dimensions for the total width (44'-8"), travelway width (26'-10"), and sidewalk width (17'-4"). The deck thickness is 9", and the sidewalk thickness is 1'-0". The haunch thickness is 3". The travelway is labeled "Travelway 1" and the sidewalk is labeled "Sidewalk". The bridge is supported by two 27x178 girders. The software interface also shows a project tree on the left with various categories like Materials, Beam Shapes, Appurtenances, Connectors, Diaphragm Definitions, Lateral Bracing Definitions, Impact / Dynamic Load Allowance, LRFDF Multiple Presence Factors, Factors, LRFDF Substructure Design Settings, Environmental Conditions, Design Parameters, SUPERSTRUCTURE DEFINITIONS, MEMBER DEFINITIONS, and BRIDGE ALTERNATIVES.

# RATIN

- DRAWING (Civil)
- Live Load
- Preliminary

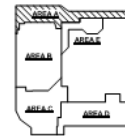


## FRAMING AND GENERAL PLAN NOTES:

1. REFER TO S1.1.0 FOR NOTES.
2. SHORE BEAMS AT THIRD-SPAN UNTIL CONCRETE ON METAL DECK REACHES 75% OF THE SPECIFIED  $f_c$ .

### FRAMING AND GENERAL PLAN NOTES:

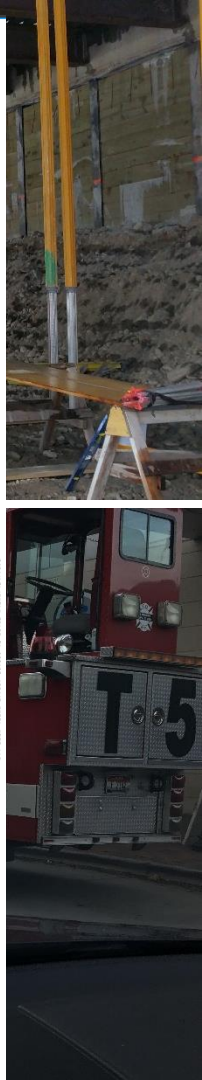
1. REBAR TIED 2' ON SPACES.
2. SHORE BEAMS AT THIRD-SPAN UNTIL CONCRETE ON METAL DECK REACHES 75% OF THE SPECIFIED  $f_c$ .



**CITY CENTER PLAZA**  
**GARDNER COMPANY**  
 377 WEST PARK STREET, SUITE 2000, DENVER, CO 80202  
 303.733.1111

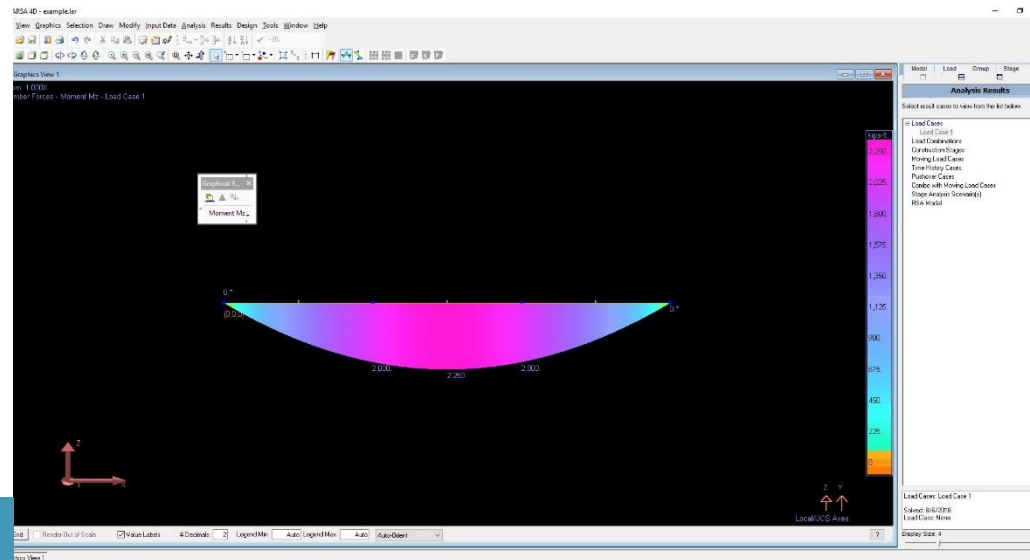
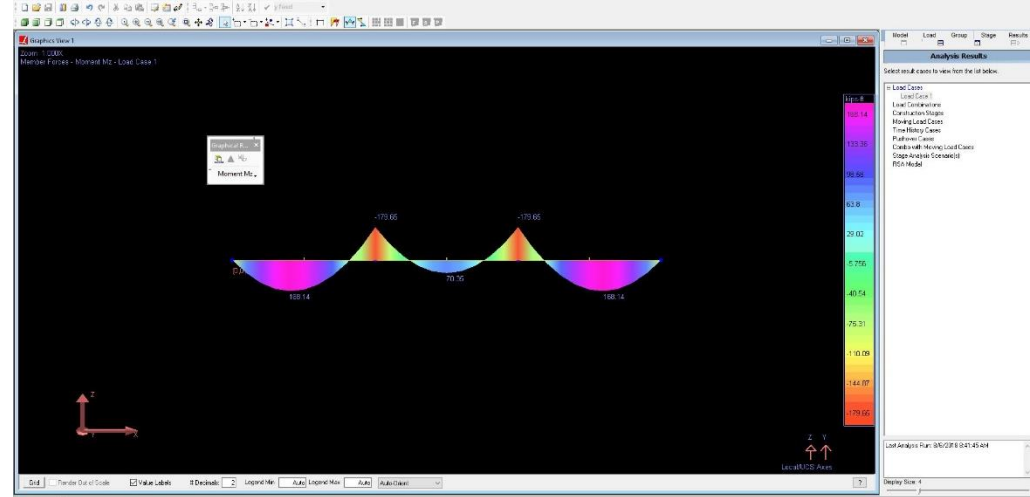
**Submittal Design Group**  
 1000 14th Street, Suite 1000, Denver, CO 80202  
 303.733.1111

**GROUND LEVEL FLOOR PLAN - AREA A**



# TEMP. SHORING

- LARSA
- Equivalent DL to match Moment



# WALLACE VIADUCT

Load rating a curved PT multicell box bridge in BrR

## Outline:

- Location/Geometry
- BrR Information
- Geometry Challenges
- Post-Tension Limitations
- Live Load Distribution Factors
- Solid Sections
- Pier Input



# LOCATION

- Carries I-90 through the town of Wallace
- Located between Spokane and Missoula
- Constructed in 1991
- Railroad under most of the structure converted to Trail of the Coeur d' Alenes



# GEOMETRY



- 8 Units – 33 spans
- Total length of 4478'
- Two ramps
- Four circular curves, with spiral transitions
- In span hinge
- Width transitions from 84' min to 145' max
- Splayed webs
- Multi-column piers

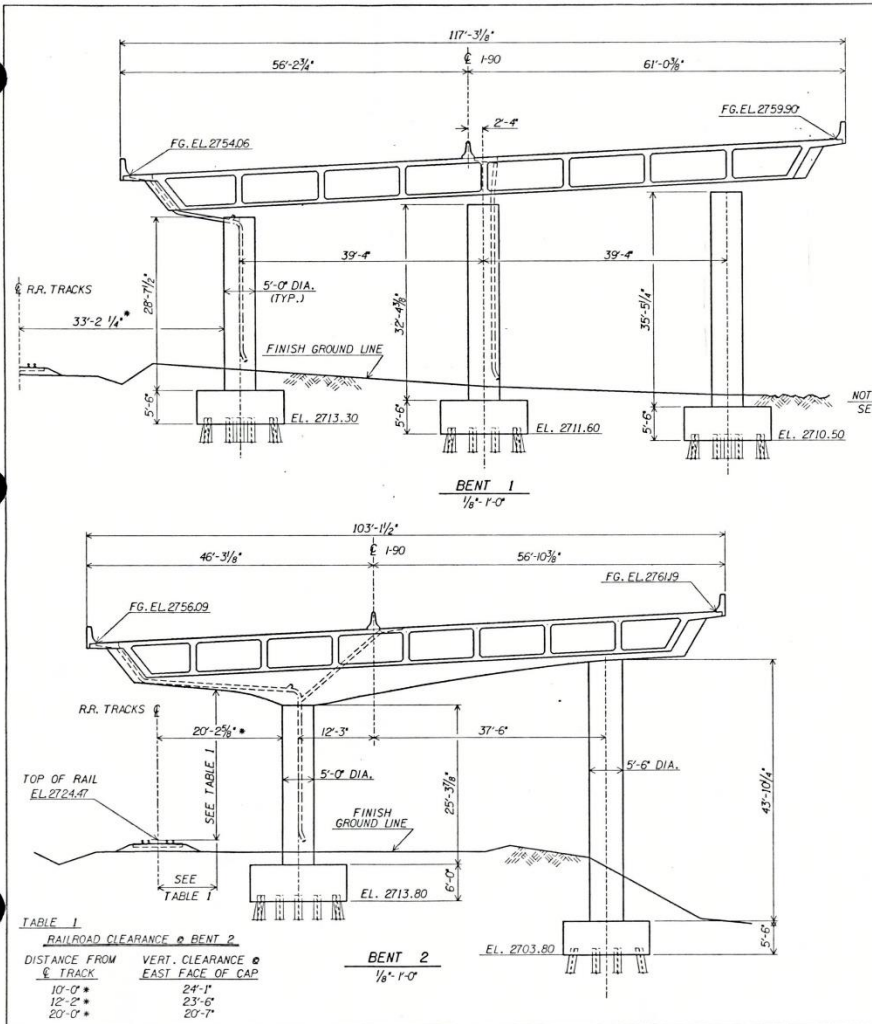


TABLE 1  
RAILROAD CLEARANCE @ BENT 2

DISTANCE FROM TRACK	VERT. CLEARANCE @ EAST FACE OF CAP
10'-0"	24'-1"
12'-2"	23'-6"
20'-0"	20'-7"



# BrR Information

- BrR Version 6.8.1 used
- Bridge designed LFR
- LRFR module was used for rating:
  - LFR module was not stable
  - LRFR module has more flexibility with live load distribution factors

Standard **LRFD**

Distribution Factor Input Method

Use Simplified Method    Use Advanced Method    Use Advanced Meth

Allow distribution factors to be used to compute effects of permit loads with routine t

Lanes Loaded	Distribution Factor (Wheels)			
	Shear	Shear at Supports	Moment	Deflection
1 Lane				
Multi-Lane				

Standard **LRFD**

Distribution Factor Input Method

Use Simplified Method    Use Advanced Method

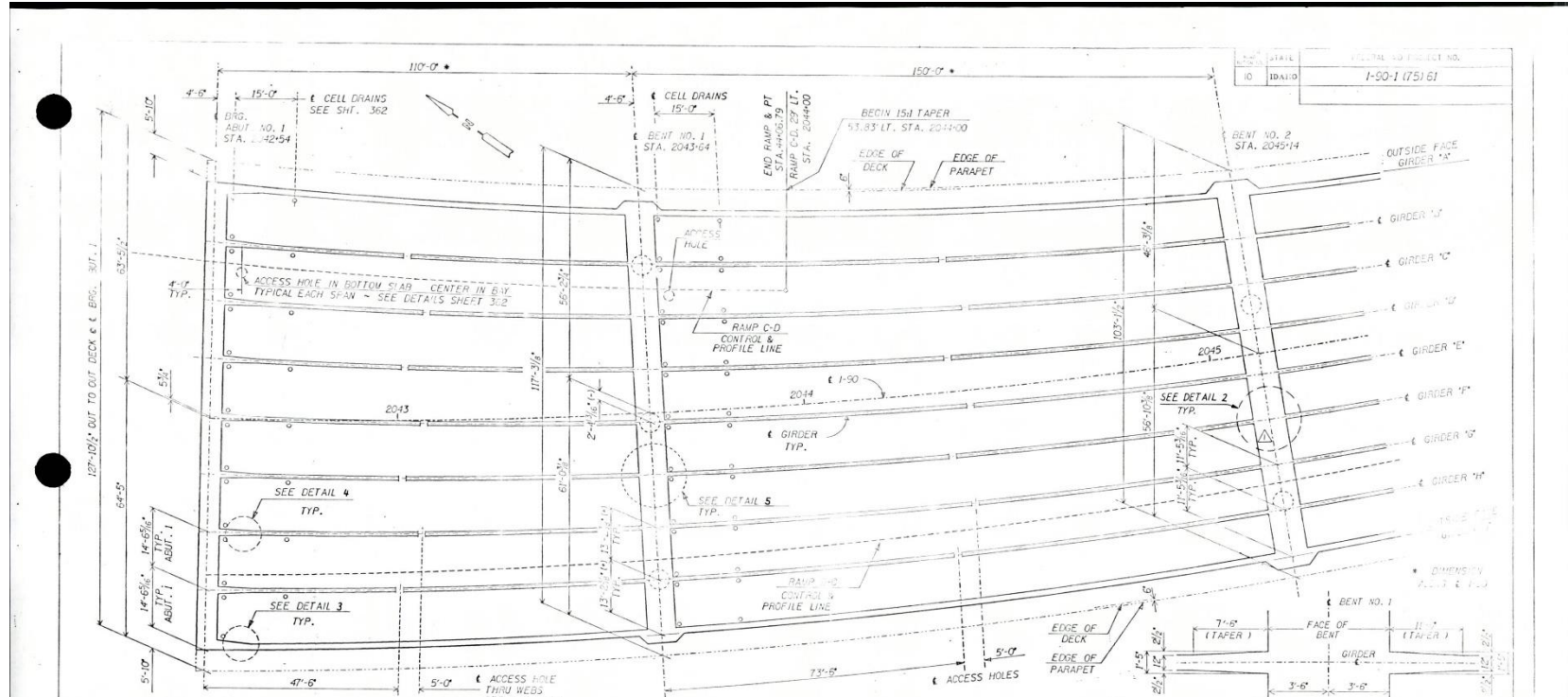
Allow distribution factors to be used to compute effects of permit loads with routine

Action: **Moment** ▼

Support Number	Start Distance (ft)	Length (ft)	End Distance (ft)	Distribution Factor (Lanes)	
				1 Lane	Multi-Lane
1 ▼	0.00	92.531	92.53	6.518	8.773
1 ▼	92.53	66.364	158.89	6.486	8.684
2 ▼	34.31	81.976	116.29	2.894	5.634
2 ▼	116.29	67.673	183.96	2.862	5.541
3 ▼	33.98	85.874	119.86	2.829	5.446
3 ▼	119.86	62.167	182.02	2.944	5.553
4 ▼	32.08	77.394	109.48	3.090	5.698

# GEOMETRY CHALLENGES

- Curved superstructure
- Non-linear splayed girder webs



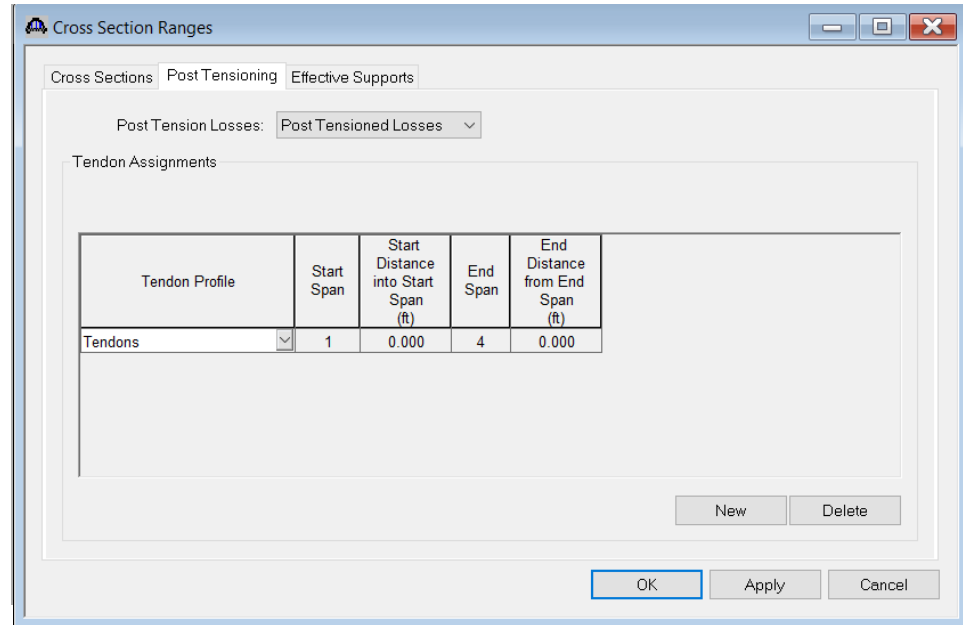
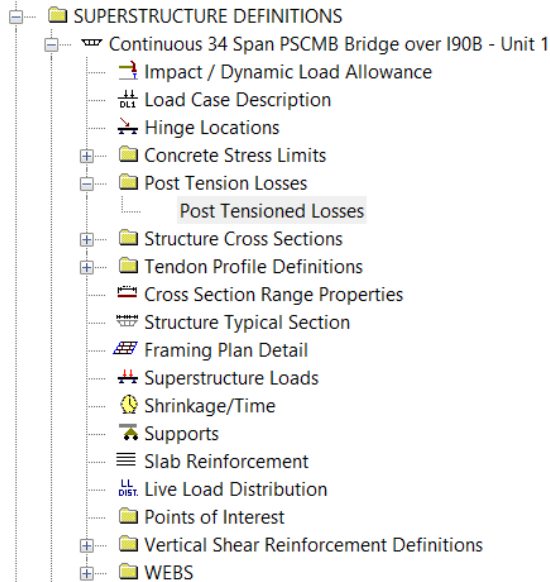
# GEOMETRY SIMPLIFICATIONS

- Curved superstructure
  - Modeled as straight, per AASHTO Art. 4.6.1.2.3
    - Central angle of individual spans less than 12 degrees
- Splayed girder webs
  - BrR does not rate individual webs for shear when girder webs are splayed.
    - AASHTO 4.6.2.2 allows cast-in-place multi-cell box girders to be analyzed as full width sections
    - All piers radial, no correction factor needed for webs in obtuse corner
- BrR can handle splayed geometry, however it has to be linear
  - Edge of deck and CL webs follow spiral and circular curves while also transitioning width, therefore edge of deck follows non-linear path
  - Span lengths and shear stirrup spacing based on middle web dimensions in design plans



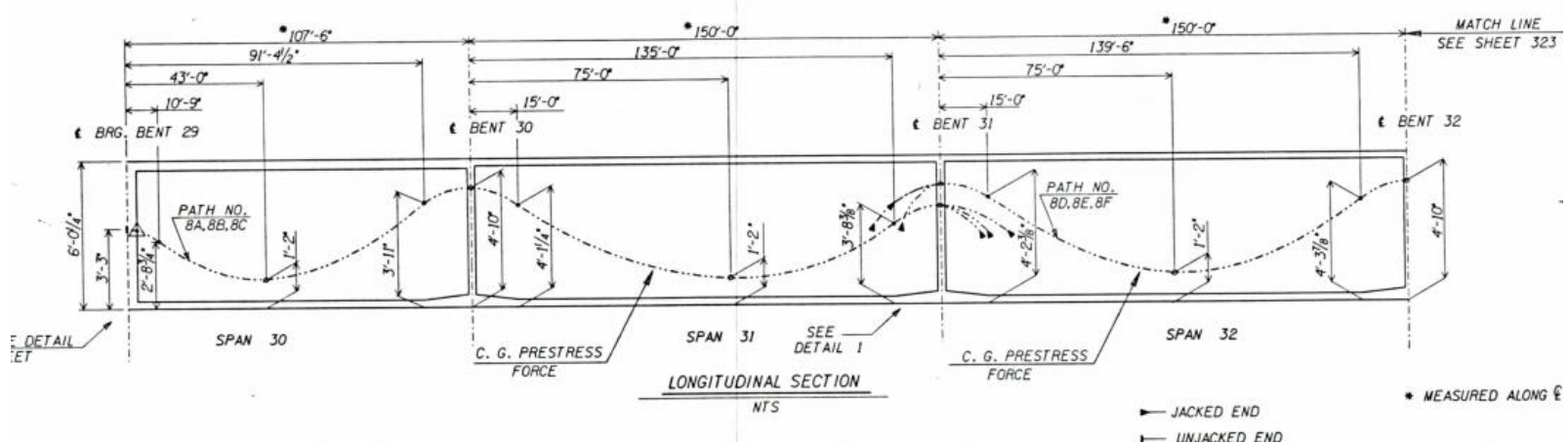
# POST-TENSION LIMITATIONS

- BrR only allows one PT Loss definition per frame
  - Limits anchor set, coefficient of friction and wobble coefficient values to be the same on every profile defined in an individual frame
  - Multiple Post Tensioned Losses can be defined, but only one can be defined in the cross section ranges window



# POST-TENSION LIMITATIONS

- Friction losses due to horizontal curvature
  - Hand calculations were ran to determine friction losses due to horizontal curvature
  - Wobble coefficient was modified to account for additional frictional losses
  - Only one wobble coefficient allowed per frame, worst case was used when more than one PT tendon profile was present





# LIVE LOAD DISTRIBUTION FACTORS

- LLDF follow current AASHTO LRFD code
  - Slight discrepancies were found in exterior and first interior web from BrR calculations
    - Interior girders and independent calculations matched
  - Independent LLDF were calculated
  - Width used for LLDF was midpoint between contraflexure points for moment, and CL span for shear

Standard LRFD

Distribution Factor Input Method  
 Use Simplified Method     Use Advanced Method

Allow distribution factors to be used to compute effects of permit loads with routing

Action: Moment

Support Number	Start Distance (ft)	Length (ft)	End Distance (ft)	Distribution Factor (Lanes)	
				1 Lane	Multi-Lane
1	0.00	78.071	78.07	0.906	0.906
1	78.07	62.178	140.25	0.885	0.885
2	30.09	86.479	116.56	0.864	0.864
2	116.56	67.769	184.33	0.841	0.841
3	33.88	82.604	116.48	1.181	1.078
3	116.48	66.046	182.53	1.179	1.053
4	31.94	92.920	124.86	1.178	1.025

Standard LRFD

Distribution Factor Input Method  
 Use Simplified Method     Use Advanced Method

Allow distribution factors to be used to compute effects of permit loads with routing

Action: Shear

Support Number	Start Distance (ft)	Length (ft)	End Distance (ft)	Distribution Factor (Lanes)	
				1 Lane	Multi-Lane
1	0.00	110.163	110.16	1.184	1.149
2	0.00	150.455	150.46	1.183	1.229
3	0.00	150.591	150.59	1.181	1.122
4	0.00	124.858	124.86	1.178	1.043



# SOLID SECTIONS

- When defining solid sections at the pier locations, BrR uses the gross area and section properties of the solid concrete section for Service III stress calculations
  - This results in an erroneous result, since  $P/A + Mc/I$  for a solid section will result in far smaller compressive stresses than the hollow section that it was designed for.
  - Recommend not using solid section check box for pier locations

Cross Section Ranges

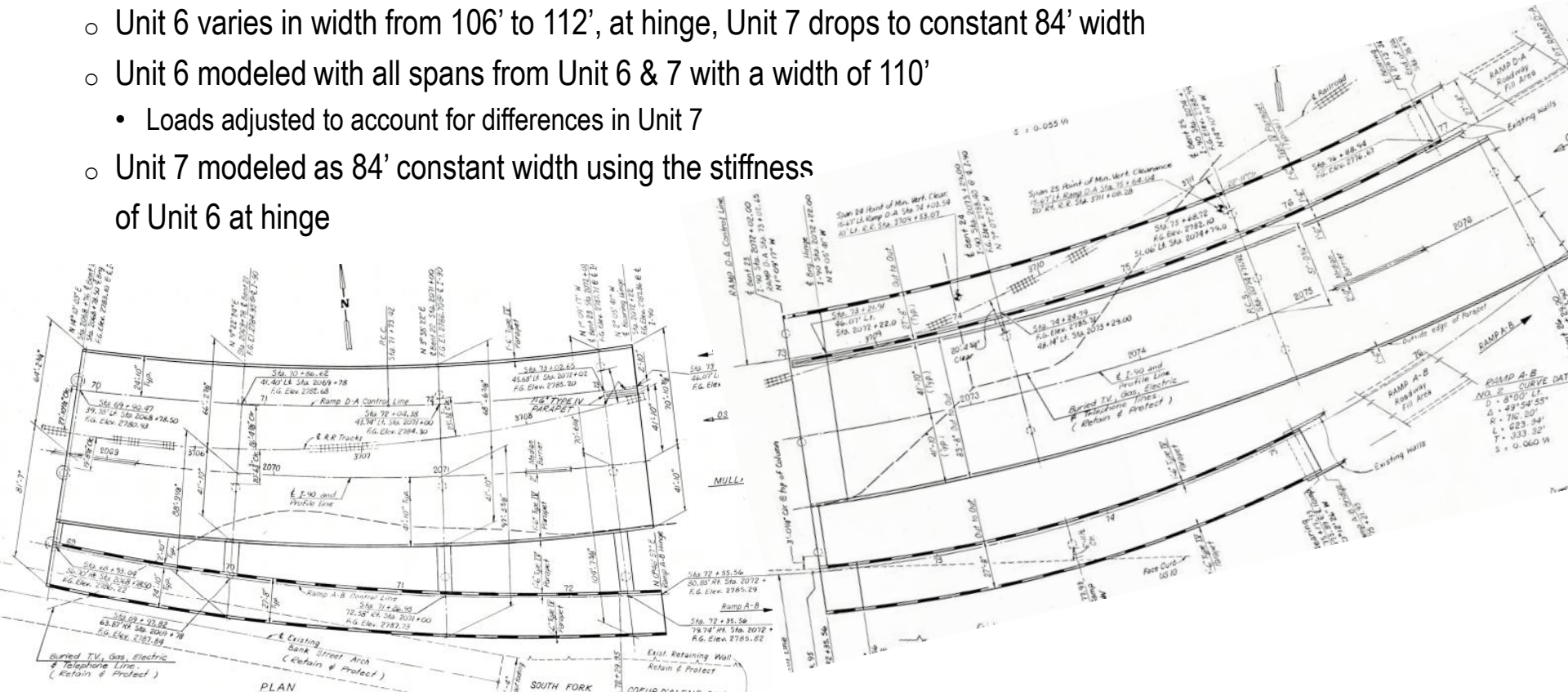
Cross Sections | Post Tensioning | Effective Supports

Left end projection:  in Right end projection:  in

Start Section	End Section	Depth Vary	Solid Section	Support Number	Start Distance (ft)	Length (ft)	End Distance (ft)
CL Abut. 1	Abut. 1 - Face	None	<input type="checkbox"/>	1	0.000	3.000	3.000
Abut. 1 - Face	Abut. 1 - Transition	None	<input type="checkbox"/>	1	3.000	8.500	11.500
Abut. 1 - Transition	Pier 1 - BS Transition	None	<input type="checkbox"/>	1	11.500	87.604	99.104
Pier 1 - BS Transition	Pier 1 - BS Face	None	<input type="checkbox"/>	1	99.104	7.500	106.604
Pier 1 - BS Face	Pier 1 - FS Face	None	<input type="checkbox"/>	1	106.604	7.000	113.604
Pier 1 - FS Face	Pier 1 - FS Transition	None	<input type="checkbox"/>	2	3.500	11.500	15.000
Pier 1 - FS Transition	Pier 2 - BS Transition	None	<input type="checkbox"/>	2	15.000	114.125	129.125
Pier 2 - BS Transition	Pier 2 - BS Face	None	<input type="checkbox"/>	2	129.125	17.250	146.375
Pier 2 - BS Face	Pier 2 - FS Face	None	<input type="checkbox"/>	2	146.375	8.000	154.375
Pier 2 - FS Face	Pier 2 - FS Transition	None	<input type="checkbox"/>	3	4.000	17.250	21.250
Pier 2 - FS Transition	Pier 3 - BS Transition	None	<input type="checkbox"/>	3	21.250	114.260	135.510
Pier 3 - BS Transition	Pier 3 - BS Face	None	<input type="checkbox"/>	3	135.510	11.000	146.510
Pier 3 - BS Face	Pier 3 - FS Face	None	<input type="checkbox"/>	3	146.510	8.000	154.510
Pier 3 - FS Face	Pier 3 - FS Transition	None	<input type="checkbox"/>	4	4.000	8.500	12.500
Pier 3 - FS Transition	Pier 4 - Transition	None	<input type="checkbox"/>	4	12.500	99.792	112.292
Pier 4 - Transition	Pier 4 - Face	None	<input type="checkbox"/>	4	112.292	8.688	120.979
Pier 4 - Face	Pier 4 - BS Brg	None	<input type="checkbox"/>	4	120.979	3.813	124.792

# HINGE

- Hinge input into BrR with simplifications:
  - Unit 6 varies in width from 106' to 112', at hinge, Unit 7 drops to constant 84' width
  - Unit 6 modeled with all spans from Unit 6 & 7 with a width of 110'
    - Loads adjusted to account for differences in Unit 7
  - Unit 7 modeled as 84' constant width using the stiffness of Unit 6 at hinge



# PIERS

- Piers input into BrR using Integral piers input in the superstructure definition

Concrete Multi-Cell Box Superstructure Definition

Definition Analysis Specs Factors Engine Control Options

Name: Continuous 34 Span PSCMB Bridge over I90B - Unit 1

Description: Bridge Key: 17247  
1.5" Concrete Wearing Surface (2015 Report):  
Start ==>  $(127.875 - (1.5)(2) - 2)(1.5)(0.150 \text{ kcf}) = 2.304 \text{ klf}$   
End ==>  $(32.896 - (1.5)(2) - 2)(1.5)(0.150 \text{ kcf}) = 1.648 \text{ klf}$   
ITD Jersey Barrier ==>  $(2)[(9)(33.5) + (2)(19)(0.5) + (2)(14.5) + (14.5 + 4.5)(7)(0.5) + (8.5)(6)](0.150 \text{ kcf}) = 0.973 \text{ klf}$   
Median Barrier ==>  $[(10 + 6)(0.5)(19) + (10 + 24)(0.5)(10) + (4.5)(24)](0.150 \text{ kcf}) = 0.448 \text{ klf}$   
Railroad Splashguards (Assume 50 plf for aluminum panel. Increase wt. 10% to account for misc. wt.) ==>

Default Units: US Customary Enter Span Lengths Along the Reference Line:

Number of spans: 4

Number of cells: 8

Span	Length (ft)
1	110.10
2	150.38
3	150.51
4	124.79

Integral piers:

Support	Integral
1	<input type="checkbox"/>
2	<input type="checkbox"/>
3	<input checked="" type="checkbox"/>
4	<input type="checkbox"/>
5	<input type="checkbox"/>

Average humidity: 55.000 %

Structure Type

Frame structure simplified definition

Integral with substructure

Consider substructure skew in FE section properties

Not integral with substructure

Post-tensioned

# PIER INPUT

- Bridge Alternative input to run multiple superstructure definitions with piers

The screenshot displays the Bridge Workspace software interface, showing the 'Bridge Alternative' and 'Superstructure' panels.

**Bridge Alternative Panel:**

Alternative Name: Continuous 34 Span PSCMB

Description Substructures

Substructure Unit Name	Station (ft)	Offset (ft)	Unit Type
Unit 1 Abut 1	0.000	-0.000	Abutment
Unit 1 Pier 1	110.104	-0.000	Pier
Unit 1 Pier 2	260.479	-0.000	Pier
Unit 1 Pier 3	410.990	-0.000	Pier
Unit 1 Pier 4	535.781	-0.000	Pier
Unit 2 Pier 4	600.000	-0.000	Pier
Unit 2 Pier 5	724.583	-0.000	Pier
Unit 2 Pier 6	874.563	-0.000	Pier
Unit 2 Pier 7	1024.500	-0.000	Pier
Unit 2 Pier 8	1133.979	-0.000	Pier
Unit 3 Pier 8	1200.000	-0.000	Pier
Unit 3 Pier 9	1299.490	-0.000	Pier
Unit 3 Pier 10	1424.469	-0.000	Pier
Unit 3 Pier 11	1549.448	-0.000	Pier
Unit 3 Pier 12	1648.938	-0.000	Pier
Unit 4 Pier 12	1700.000	-0.000	Pier
Unit 4 Pier 13	1824.510	-0.000	Pier
Unit 4 Pier 14	1974.510	-0.000	Pier
Unit 4 Pier 15	2124.448	-0.000	Pier
Unit 4 Pier 16	2248.719	-0.000	Pier
Unit 5 Pier 16	2300.000	-0.000	Pier
Unit 5 Pier 17	2420.287	-0.000	Pier
Unit 5 Pier 18	2566.276	-0.000	Pier
Unit 5 Pier 19	2712.521	-0.000	Pier
Unit 5 Pier 20	2833.724	-0.000	Pier
Unit 6 Pier 20	2900.000	-0.000	Pier
Unit 6 Pier 21	2998.500	-0.000	Pier
Unit 6 Pier 22	3119.177	-0.000	Pier
Unit 6 Pier 23	3219.990	-0.000	Pier

**Superstructure Panel (Unit 1):**

Superstructure Name: Unit 1

Description Alternatives Vehicle Path Engine Substructures

Select the substructure supports:

Support	Substructure Support
1	Unit 1 Abut 1
2	Unit 1 Pier 1
3	Unit 1 Pier 2
4	Unit 1 Pier 3
5	Unit 1 Pier 4

New Delete

**Superstructure Panel (Unit 2):**

Superstructure Name: Unit 2

Description Alternatives Vehicle Path Engine Substructures

Select the substructure supports:

Support	Substructure Support
1	Unit 2 Pier 4
2	Unit 2 Pier 5
3	Unit 2 Pier 6
4	Unit 2 Pier 7
5	Unit 2 Pier 8

New Delete

# SUMMARY

- Location/Geometry
- BrR Information
- Geometry Challenges
- Post-Tension Limitations
- Live Load Distribution Factors
- Solid Sections
- Pier Input



Questions?



